

## **DRAINAGE STRATEGY**

# PROPOSED RESIDENTIAL DEVELOPMENT AT THE SITE ADJECENT TO CROWN STREET, GWALCHMAI

JUNE 2022 Suitability S1 Revision P02



#### **Prepared on Behalf of:**

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### **1.0 Introduction**

#### 1.1 Project Background

- 1.1.1 Cadarn Consulting Engineers Ltd have been appointed by AMP Construction and Groundworks Ltd to provide a drainage strategy, for both surface water and foul, for the proposed development at the site adjacent to Llain Rallt, Gwalchmai (National Grid Reference: SH 39404 75966). Refer to the drawing enclosed in Appendix A for the proposed site location plan.
- 1.1.2 Cadarn Consulting Engineers Ltd reserve the right to undertake further investigation into the adequacy of the recommendations made within this report, if works on site have not commenced within twelve months of the issuing of this report.

#### 1.2 Scope of Proposed Drainage Strategy

- 1.2.1 This report aims to provide a suitable drainage strategy for the discharge of surface water and foul effluent generated by the proposed development.
- 1.2.2 The purpose of the calculations enclosed within this report and accompanying details are to produce a drainage layout that complies with the relevant legislation of the Tan 15, CIRIA C753 '*The SuDS Manual*' and Approved Document H of the Building Regulations 2010.

#### **1.3 Proposed Development**

1.3.1 AMP Construction and Groundworks Ltd are proposing to construct a residential housing development consisting of 33 properties with a new Highway network separated into two phases on a greenfield parcel of land located in Gwalchmai. The proposed site layout is illustrated within Appendix B.

#### 1.4 Site Hydrology

1.4.1 The site generally falls south easterly away from the Crown St highway, down towards the boundary with the adjacent field.

#### **1.5** Existing Land Drainage Features / Watercourses.

1.5.1 There are no existing land drainage features within the site although there is an existing land drainage feature located within the adjacent field to the southeast of the site.

#### **1.6 Existing Nearby Drainage**

- 1.6.1 As noted in Section 1.5, an onsite investigation within the site and surrounding area has identified a land drainage feature located within the field adjacent to the south-eastern boundary of the development site, running along the boundary of the adjacent field and Gwalchmai football field. The invert of this land drainage ditch has been identified as being 1m lower than the lowest point of the proposed development site and is separated from the site by a soil bund. The existing site drainage arrangement has been illustrated on the attached layout contained within Appendix C.
- 1.6.2 The Dwr Cymru / Welsh Water (DCWW) apparatus map contained within Appendix D indicates there is a 150mm vitrified clay (VC) foul sewer located within the Crown St Highway which serves the existing Maes Meurig housing estate located to the south of the site of the proposed development. This foul sewer then proceeds to continue in a southeastern direction to the sewage treatment plant.

## 2.0 Design Criteria

- 2.1.1 The following design criteria will apply to the surface water run-off and foul discharge design for the site:
  - Approved Document H, Building Regulations.
  - BRE Digest 365;
  - BS EN 752:2017;
  - CIRIA C753 'The SuDS Manual' 2015;
  - DEFRA / Environment Agency 'Preliminary Rainfall Runoff Management for Developments' Technical Report;
  - Discharge Units from BS EN 12056: Part 2;
  - Flood & Water Management Act 2010;
  - Highways Act 1991;
  - Institute of Hydrology Report (IHR) 124;
  - Land Drainage Act 1991;
  - Modified Rational Method;
  - Sewers for Adoption 7<sup>th</sup> Edition;
  - Statutory standards for sustainable drainage systems designing, constructing, operating and maintaining surface water drainage systems
  - Technical Advice Note (TAN) 15: Development and Flood Risk;
  - Wallingford Procedure;
  - Water Industries Act 1991.

## 3.0 Surface Water Drainage Design

#### 3.1 Guiding Principles

3.1.1 The disposal of surface water has been designed in strict accordance with the provision of TAN 15, the Flood and Water Management Act 2010 and other best practice documents, such as CIRIA C753 'SuDS Manual' 2015.

#### 3.2 Method of discharge

- 3.2.1 In accordance with the SuDS Manual 2015, surface water should be managed and discharged from a new development in line with the following hierarchy:
  - Priority level 1: Re-use of water;
  - Priority level 2: Infiltration into ground;
  - Priority level 3: Discharge to a water body;
  - Priority level 4: Discharge to a surface water run-off drain;
  - Priority level 5: Discharge to a combined surface water run-off and foul drain.
- 3.2.2 Re-use of surface water cannot be considered as the soul method of surface water disposal, paragraph G1.1 The Statutory SuDS Guidance for Wales states "As much of the runoff as possible (subject to technical or cost constraints) should be discharged to each destination before a lower priority destination (level) is considered." Therefore, attempts must be made to reuse as much surface water as possible. Therefore, it is proposed to provide above ground water butts on one rainwater downpipes to the rear of each property to allow the residence of the development the ability to water the plants and rear garden areas.
- 3.2.3 Infiltration testing has been undertaken by '*Cadarn*' as part of a phase I & II site investigation (this report is not included as part of this strategy due to the size, however this can be issued upon request). A summary of the report is provided below.

- Porosity testing was undertaken within two trial pits located within the site, a drop in water level of 0.10m 0.12m was recorded within the first 1.5 hours (90 minutes) although slowed to a holt.
- BRE 365 requires three tests to be undertaken within each trial hole however due to the slow rate of infiltration recorded this was not possible.
- 3.2.4 The report concludes that the use of infiltration systems such as soakaways for the disposal of surface water run-off generated from the proposed development is not suitable, however as noted in **Section 3.2.2** attempts should be made despite the poor infiltration rate, therefore all proposed attenuation features are to be wrapped in a non-woven geotextile to allow migration of surface water into the ground.
- 3.2.5 As noted in **Section 1.5** there is land drainage feature located 20m east of the sites boundary therefore it is proposed to communicate flows from the site to this watercourse.

#### 3.3 Climate Change

- 3.3.1 TAN 15 states that an allowance for climate change should be provided within the on-site attenuation, without specifying what allowance should be made. The NPPF, which is the English equivalent of TAN 15, does however provide guidance derived from DEFRA FCDPAG3 'Economic Appraisal Supplementary Note to Operating Authorities – Climate Change Impacts' October 2006 (see Table 1). This document considers the effects of climate change for different design criteria.
- 3.3.2 The proposed development will have a design life of 100 years; based on the NPPF's guidance, the development therefore requires an allowance of 30% for climate change to be applied to the peak rainfall intensity.

#### Table 1. Climate Change Requirements.

Donomotor	1990 to	2025 to	2055 to	2085 to
rarameter	2025	2055	2085	2115
Peak Rainfall Intensity	+5%	+10%	+20%	+30%
Peak River Flow	+10%	+20%		
Offshore Wind Speed	+	+5% +10%		%
Extreme Wave Height	+	+5% +10%		%

#### 3.4 Design Philosophy

Based upon existing site information and the details of the proposed development, an assessment of the site run-off has been undertaken utilising the 'Flow' hydraulic modelling package (refer to **Appendix E**). This has enabled the existing greenfield run-off flows to be assessed and quantified, in accordance with IHR 124.

3.4.1 The tables below summarise the existing effective areas with their corresponding run-off coefficients, as per the Wallingford procedure, IHR 124.

**Table 2.** Existing Greenfield Areas.

Surface	Total Area	Coefficient	Effective Area
Grass	8,206.316 m <sup>2</sup>	0.35	2872.211 m <sup>2</sup>
Surface	2,003.308 m <sup>2</sup>		2872.211 m <sup>2</sup>

3.4.2 The areas provided within **Table 2** are provided on the existing greenfield area drawing contained within **Appendix F.** Reference should be made to the attached hydraulic model output calculations containing the run-off rates for the site (**Appendix E**), which are summarised as follows:

#### Table 3. Greenfield Run-off rates for different return periods.

Reference	1 in 1 Year	1 in 30 Year	1 in 100 Year
Greenfield Rate	5.5 l/s	11.2 l/s	13.5 l/s

3.4.3 The Statutory SuDS guidance for Wales requests all development to be restricted to minim greenfield run-of rates.

3.4.4 The exact greenfield run-off rates discharge rates connect be achieved without the need for a second hydrobrake within the chamber therefore a slight betterment for storm events over the 1:1 year return period.

#### 3.5 Method of Storage

- 3.5.1 Surface water run-off generated from all proposed hardstanding areas for the 1 in 100-year return period plus an allowance of 30% for climate change is to be attenuated onsite within a below ground storage structures and the below ground piped drainage network. Raingardens and porous paving are also included as part of the design however have not been accounted for within the attenuated volume but will help to reduce the rate of flow through the site. This section of the report should be read in conjunction with the proposed on-site drainage arrangement enclosed in **Appendix G**.
- 3.5.2 The proposed measured hardstanding areas accounted for within each soakaway summarised within **Table 4** and illustrated on the proposed impermeable area plan contained within **Appendix H.**

Surface	Total Area	Coefficient	Effective Area
Houses	1,646.241 m <sup>2</sup>	1.00	1,646.241 m <sup>2</sup>
Path	1,189.051 m <sup>2</sup>	1.00	1,189.051 m <sup>2</sup>
Swale	429.651 m <sup>2</sup>	1.00	429.651 m <sup>2</sup>
Parking Bays	864.832 m <sup>2</sup>	1.00	864.832 m <sup>2</sup>
Road	1,461.139 m <sup>2</sup>	1.00	1,461.139 m <sup>2</sup>
Grass	2,615.401 m <sup>2</sup>	0.35	915.390 m <sup>2</sup>
Total	8,206.316 m <sup>2</sup>		6,506.304 m <sup>2</sup>
Total – Grass Removed	5,590.915 m <sup>2</sup>		5,590.915 m <sup>2</sup>

**Table 4.** North section of site.

3.5.3 An Additional 10% is applied to the effective area (grass removed) to account for urban creep, therefore the total effective area is: 6,150.035 m<sup>2</sup>

3.5.4 The 'Flow' hydraulic modelling package output is contained within AppendixE which indicates sufficient storge is provided within the site.

- 3.6.1 The SuDS Manual 2015 requires appropriate measures to be in place for the maintenance of surface water drainage systems and sustainable drainage features.
- 3.6.2 The maintenance schedule shown in **Tables 5 & 6** have been derived in strict accordance with the SuDS Manual 2015 and from a risk-assessed approach during the design stage. These schedules are not exhaustive and should be reassessed at regular intervals to determine if any additional maintenance requirements are required to preserve the performance and condition of the site drainage system.
- 3.6.3 All surface water drainage located within the highway will be adopted and maintained by the highway authority under section 38 of the Highways Act 1980. The remainder of the drainage network (besides private drainage within an individual property boundary) is to be adopted and maintained by the SAB. The maintenance schedule for each component of the proposed drainage network is contained within Tables 5 & 6.
- 3.6.4 Provided preventive maintenance measures are undertaken in accordance with the frequencies recommended in Tables 5 & 6, the need for corrective maintenance should rarely arise.
- 3.6.5 Maintenance activities should be detailed in the Principal Contractor's Health and Safety Plan and Risk Assessments and should be updated on a regular basis to ensure the continued performance and long-term condition of the drainage system.

**Table 5.** Operation and maintenance requirements for filter drains in line with table16.1 of the CIRIA C753 'The SuDS Manual 2015.

Maintenance Schedule	<b>Required Action</b>	Typical Frequency
Monitoring	Inspect filter drain surface, inlet/outlet pipework	Six Monthly
	and control systems for blockages, clogging,	
	standard water, and structural damage.	
	Inspect pre-treatment systems, inlets, and	Six Monthly
	perforated pipework for silt accumulation, and	
	establish appropriate silt removal frequencies.	
	Inspect filter drain for tree roots encroaching	
	sides.	
Regular	Remove litter (including leaf litter) and debris	Six Monthly
Maintenance	from filter drain surface, access chambers and	
	pre-treatment devices	
	Inspect filter drain surface, inlet/outlet pipework	Six Monthly
	and control systems for blockages, clogging,	
	standard water, and structural damage.	
	Inspect pre-treatment systems, inlets, and	Six Monthly
	perforated pipework for silt accumulation, and	
	establish appropriate silt removal frequencies.	
	Remove Sediment from pre-treatment devices	Six Monthly or as
		required.
Occasional	Remove or control tree roots where they are	As required.
Maintenance	encroaching the side of the filter drain, using	
	recommended methods (eg NJUG, 2007 or SB	
	3998:2010)	
	In locations where high level of pollutants are	As required.
	anticipated, remove surface geotextile and	
	replace or wash, and replace overlaying filter	
	medium.	
	Clear perforated pipework of blockages	As required.
Remedial	Reconstruct filter drain (replacing the all	As required.
Actions/	geotextile/membrane and granular medium) if	
Corrective	not maintained in line with the above	
Maintenance	recommendations and is completely clogged up.	

**Table 6.** Operation and maintenance requirements for Swale in line with table 17.1 ofthe CIRIA C753 'The SuDS Manual 2015.

Maintenance	Required Action	Typical Frequency	
Schedule			
Monitoring	Inspect Inlets & Outlets for blockages and clear	Quarterly	
	if required.		
	Record rate of sediment accumulation and	Quarterly for first	
	establish appropriate silt removal	year, then annually or	
	frequency/maintenance plan.	as required.	
	Inspection of check dams to ensure they are	Annually.	
	intact are holding water back effectively		
Regular	Removal of litter and debris.	as required.	
Maintenance			
	Cutting Grass in and around swale.	as required (Spring –	
		before nesting season	
		and autumn)	
	Manage vegetation and removal nuisance plants.	Two monthly for 6	
		months, then	
		annually.	
	Remove sediments from inlets and outlets.	Annually or as	
		required.	
Occasional	Reseed areas of poor vegetation growth.	As required.	
Maintenance			
Remedial	Repair erosion or other damage by reseeding or	As required.	
Actions/	re-turfing.		
Corrective	Repairing check dams if damaged.	As required.	
Maintenance	Repair/rehabilitation of inlets and outlets.	As required.	
	Relevel uneven surfaces and reinstate design	As required.	
	levels.		

**Table 7.** Operation and maintenance requirements for bioretention systems (Rain-<br/>gardens) in line with table 18.3 of the CIRIA C753 'The SuDS Manual 2015.

Maintenance	Paguirad Action	Typical Frequency	
Schedule	Required Action	Typical Frequency	
Monitoring	Inspect infiltration surfaces for silting and	Six Monthly.	
	ponding, record de-watering time of the facility		
	and assess standing water levels in under drain		
	(if appropriate) to determine if maintenance is		
	necessary.		
	Check operation of under drains	Annually.	
	Assess Plants for disease, infection, poor	Six monthly.	
	growth, invasive species etc. and replace is		
	necessary		
	Inspect inlets and outlets for blockages	Six monthly.	
Regular	Removal of litter and debris and weeds	Quarterly	
Maintenance			
	Replace any plants, to maintain planting density.	as required.	
Occasional	Infill any holes or scour in the filter medium,	As required.	
Maintenance	improve erosion protection if required.		
	Repair minor accumulation of silts by raking	As required.	
	away surface mulch, scarifying surface of		
	medium and replacing mulch,		
Remedial	Remove and replace filter medium and	As required.	
Actions/	vegetation above.		
Corrective			
Maintenance			

**Table 8.** Operation and maintenance requirements for porous paving in line with table20.15 of the CIRIA C753 'The SuDS Manual 2015.

Maintenance Schedule	Required Action		Typical Frequency
Monitoring	Initial inspection		Monthly for first 3 months
	Inspect for evidence of poor operation		Annually or as required.
	and/or weed growth and tak	e appropriate	
	action if required.		
	Inspect silt accumulation ra	tes and	Annually.
	establish appropriate brushi	ng	
	frequencies.		
Regular	Brushing and vacuuming	Annually after	autumn leaf fall, or reduce
Maintenance	over whole surface,	frequency as re	equired based on site-specific
	(standard cosmetic sweep	observations of	f clogging or manufacturers
	over whole surface).	recommendation	ons – pay close attenuation to
		areas where wa	ater runs onto porous areas
		from adjacent	impervious areas as this is
		most likely to	collect the most sediments.
Occasional	Removal of weeds or managed	gement using	As required.
Maintenance	glyphospate applied directly	y into weeds	
	by an applicator rather than	spraying.	
	Stabilise and mow contribu	ting and	As required.
	adjacent areas.		
Remedial	Remediate any landscaping	which,	As required.
actions	through vegetation mainten	ance or soil	
	slip, has been raised within	50mm of the	
	level of the paving.		
	Remedial work to any depre	essions, rutting	As required.
	and cracked or broken bloc	ks considered	
	detrimental to the structural	performance	
	or a hazard to users and rep	lace lost	
	jointing material.		
	Rehabilitation of surface an	d upper	Every 10 to 15 years or as
	substructure by remedial sw	veeping	required (if infiltration
			performance is reduced due
			to clogging)

Maintenance Schedule	Required Action	Typical Frequency
Monitoring	Inspect using CCTV drain surveys to ensure they	Every 5 years (or as
	are in good condition and operating as designed.	required)
	Inspect chambers to ensure they are in good	Annually
	condition and that accumulation of sediment,	
	debris etc. is not preventing them from operating	
	as designed.	
Regular	Remove any accumulation of silt, sediment,	Bi-annually
Maintenance	leaves, debris etc.	
Occasional	High-pressure water jet for removal of silt builds	Every 5 years (or as
Maintenance	up and avoid blockages, particularly at bends or	required)
	changes in direction.	
Remedial	High-pressure water jet to remove blockages.	As Required.
Actions/		
Corrective		
Maintenance		

\*Hydrobrake flow control device to be maintained in accordance with hydro internationals recommendations.

#### 3.7 Surface Water Treatment

3.7.1 In accordance with the SuDS Manual 2015, Table 26.2, commercial yards and delivery areas are classified as having a 'medium' pollution hazard level. Table
12 shows the pollution hazard indices for the land use.

Table 10.	Pollution	Hazard	Indices
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Land Use	Pollution Hazard Level	Total Suspended Soils (PMI <sub>TSS</sub> )	Hydrocarbons (PMIнм)	Heavy Metals (PMI <sub>PAH</sub> )
Residential Roofs	Very Low	0.2	0.05	0.2
Low Traffic Roads / Residential Car Parks	Low	0.5	0.4	0.4

3.7.2 The SuDS Manual 2015 also provides pollution mitigation indices for different SuDS drainage features, as detailed in **Table 13**. The increase in surface water pollution resulting from the hardstanding areas during normal conditions is likely to be negligible. However, this risk increases during rainfall events of greater intensity. As previous noted the proposed design includes two shallow planted swales, rain gardens and soakaways (the raingardens and planted swales can be classified as bioretention systems), the proposed soakaways cal also be classified as filter drains due to their granular nature all surface water run-off from the site passing through at least two of these systems.

	Pollution Mitigation Indices				
SuDS Component	Total Suspended Soils (PMI <sub>TSS</sub> )	Hydrocarbons (PMI <sub>HM</sub> )	Heavy Metals (PMIPAH)		
Filter Strip	0.9	0.8	0.7		
Filter Drain	0.6	0.8	0.7		
Swale	0.7	0.6	0.4		
<b>Bioretention System</b>	0.8	0.8	0.8		
Porous Paving	0.2 0.3		0.3		
Infiltration Basin	0.05	0.05	0.05		
Detention Basin	0.7	0.7	0.6		
Pond	0.7	0.7	0.5		
Wet Land	0.8	0.8	0.8		
Proprietary Treatment Systems	These must demonstrate that they can address each of the contaminant types to acceptable levels for frequent events up to approximately the 1 in 1-year return period event, for inflow concentrations relevant to the contributing drainage.				

#### Table 11. Pollution Mitigation Indices

3.7.3 The sum of the highlighted mitigation values (Bold) with in each column of Table 12 surpass the hazard values within Table 13, therefore suitable treatment of the surface water run-off from the site is provided, as indicated within Table 14.

#### 4.1 Method of Discharge

- 4.1.1 Design of the foul sewers included within the proposal has been carried out in accordance with BS EN 12056 Part 2, Approved Document H of the Building Regulations 2010 and other best practice documents, such as the 'Sewers for Adoption' 7<sup>th</sup> edition. In accordance with Approved Document H, the preference in terms of the priority for discharging foul effluent is to discharge into a public foul sewerage system.
- 4.1.2 As previously detailed, there is a Ø150mm gravity foul sewer located within the highway located to the north of the proposal and within the Maes Meurig housing estate. As identified within the DCWW pre planning advice PPA the point of connection to the sewer has been confirmed as being to the south of the site at the chamber within the adjoining playground, however due to site levels a connection is to be formed slightly downstream of the originally suggested location. The hierarchy outlined in Approved Document H of the Building Regulations 2010 can therefore be satisfied by connecting into this sewer.
- 4.1.3 The design of the foul drainage system, along with the surface water system, for the proposed development is illustrated in the drawing enclosed in Appendix G.

#### 4.2 Drainage System Maintenance

4.2.1 The site's foul drainage system shall be adopted by the Local Water Company, Dŵr Cymru / Welsh Water, who shall be responsible for maintaining the system in strict accordance with their own standards and any relevant codes and regulations. The sections of proposed foul drainage which are serving a single property and within the boundary of said property will remain private and the responsibility of the homeowner.



# APPENDICES



## **APPENDIX A** Site Location Plan



Cadarn Consulting Engineers Ltd



## APPENDIX B Proposed Site Layout



Peidiwch a chymryd mesuriadau graddfa oddi ar y dyluniad hwn Os yn amau - gofynnwch Do not scale from this drawing lf in doubt - ask

Mae'r dyluniad hwn yn hawlfraint Penseiri Russell-Hughes ac ni chaniateir ei gopio neu ei atgynhyrchu heb ganiatad

This drawing is the copyright of Russell-Hughes architects and ust not be copied or reproduced without

Newidiadau - Amendment REV A - 25|03|2022 - SCHEME REDUCED TO 20 DWELLINGS REV B - 22|04|2022 - SCHEME AMENDED TO ADD I BEDROOM APARTMENTS REV C - 23J05J2022 - FULL SCHEME INCLUDED ON DRAWING REV G - 16/6/2022 - LAYOUT AMENDED REV J - 17/6/2022 - LAYOUT AMENDED

#### Cynllun - Job

RESIDENTIAL DEVELOPMENT ON LAND ADJACENT TO LLAIN RALLT, GWALCHMAI Dyluniad - Drawing

PROPOSED SITE PLAN

Rhif Dyluniad - Drawing No. 2947:21:3 Graddfa - Scale I:500 A3 Dyddiad - Date May 2022



56 Bridge Street, Llangefni, Ynys Mon LL77 7HH

PAVED AREAS **GARDENS / TURFED AREAS** AREAS DESIGNATED FOR SUDS DRAINAGE

TARMACADAM FOOTWAY - 4.8m x 2.4m TARMACADAM PARKING BAYS

- 5.5m WIDE TARMACADAM ESTATE ROAD

2.4m x 70m VISIBILITY SPLAYS

SITE BOUNDARY - PHASE 2

SITE BOUNDARY - PHASE I

**GRAND TOTAL - 11 RESIDENTIAL UNITS** 

4 No. - I BEDROOM APARTMENTS

I No. - 4 BEDROOM 2 STOREY DWELLINGS

2 No. - 3 BEDROOM 2 STOREY DWELLINGS

4 No. - 2 BEDROOM 2 STOREY DWELLINGS

HOUSING MIX - PHASE 2 (TBC)

**GRAND TOTAL - 22 RESIDENTIAL UNITS** 

8 No. - I BEDROOM APARTMENTS

2 No. - 4 BEDROOM 2 STOREY DWELLINGS

6 No. - 3 BEDROOM 2 STOREY DWELLINGS

6 No. - 2 BEDROOM 2 STOREY DWELLINGS

HOUSING MIX - PHASE I



## **APPENDIX C** Topographic Survey and Drainage Layout





## APPENDIX D Dŵr Cymru / Welsh Water Apparatus Map







apparatus.

WU298565. found to be Asbestos Cement (AC) or Pitch Fibre (PF). It is therefore advisable that the possible presence of AC or PF pipes be anticipated and considered as part of any risk assessment prior to excavation



## **APPENDIX E** 'Causeway Flow' Hydraulic Model

Cadarn Consulting Enginee			File: 09422_FLOV	V_MODEL .pfc	Page 1		
			Network: Storm	Network			
CAUSEVVAI 💕			Byron Thorne				
-			24/06/2022				
		Design S	Settings				
		_	-				
Rainfall Methodol	ogy FSR	Max	ximum Time of Co	oncentration (mi	ns) 30.00		
Return Period (ye	ars) 100		Maximum Rainfall (mm/hr) 50.0				
Additional Flow	(%) 30		Minimum Velocity (m/s) 1.00				
FSR Reg	jion England and W	Vales		Connection Ty	/pe Level Soffits		
M5-60 (n	nm) 17.000		Minimum Ba	ackdrop Height (	(m) 0.200		
Rati	o-R 0.300		Preferre	ed Cover Depth (	(m) 1.200		
	CV 0.750		Include Int	ermediate Grou	ınd √		
Time of Entry (m	ins) 5.00		Enforce best pr	ractice design ru	lles √		
		Simulation	Settings				
			<u> </u>				
Rainfall M	ethodology FSR		Drain	Down Time (mi	ns) 240		
	FSR Region Englan	nd and Wales	Wales Additional Storage (m <sup>3</sup> /ha) 20.0				
Ν	/15-60 (mm) 17.000	)	Check	c Discharge Rate	(s) √		
	Ratio-R 0.300			1 year (l	/s) 5.5		
			30 year (l	/s) 11.2			
	Winter CV 0.840			100 year (l	/s) 13.5		
Analysis Speed Normal			Check	Discharge Volu	me √		
Skip Steady State x			100 year +30	% 360 minute (n	n³) 362		
		Storm Du	urations				
15 30 6	0 120 180	240	360 480	600 720	960 1440		
R	eturn Period Clima	te Change	Additional Area	Additional Flow	N		
	(years) (	(CC %)	(A %)	(Q %)			
	1	30	10		0		
	30	30	10		0		
	100	30	10		0		
	<u>Pre</u> -	-development	t Discharge Rate				
Site Makeun Brownf			Time of Conce	entration (mins)			
Brownfield Method MRM			Retterment (%) 0				
Contributing Area (ba) 0.610				0.1  year (1/c)	v		
				$Q \perp y = (1/5)$			
	riivir (%)	100		(1/5)			
	CV	0.750		Q 100 year (I/S)			

#### Pre-development Discharge Volume

Site Makeup	Brownfield	CV	0.750	Betterment (%)	0
Brownfield Method	MRM	Return Period (years)	100	PR	0.750
Contributing Area (ha)	0.610	Climate Change (%)	30	Runoff Volume (m <sup>3</sup> )	362
PIMP (%)	100	Storm Duration (mins)	360		



## **APPENDIX F** Existing Greenfield Impermeable Area Plan





## APPENDIX G Proposed Drainage Layout



NOTES CONTINUED	NOTES
23. BACKFILLING TO PIPE TRENCHES BENEATH LANDSCAPED AREAS TO BE SELECTED EXCAVATE MATERIAL FREE FROM LARGE STONES GREATER THAN 0mm, LUMPS OF CLAY OVER 100mm, ANY TIMBER, FROZEN MATERIAL OR VEGETATION MATTER UP TO FROMATION / GROUND LEVEL FROM THE TOP OF THE SPECIFIED PIPE SURROUND (WELL COMPACTED IN 150 cm LAVERS)	<ol> <li>ALL LEVELS IN METERS UNLESS NOTED OTHERWISE ON DRAWING.</li> <li>ALL DIMENSIONS AND LEVELS TO BE CHECKED ON SITE PRIOR TO UNDERTAKING ANY WORKS, ORDERING MATERIALS OR FABRICATING ANY COMPONENTS.</li> </ol>
<ul><li>24. GRANULAR MATERIAL NOMINAL SIZE 20mm SINGLE SIZED OR 14mm TO 5mm GRADED.</li></ul>	3. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT ENGINEER'S AND ARCHITECT'S DRAWINGS AND RELEVANT SPECIFICATION CLAUSES.
25. BACKFILL MUST NOT BE PLACED ON CONCRETE BEDDING OR SURROUND UNTIL THE CONCRETE COMPRESSIVE STRENGTH HAS REACHED 15N/mm <sup>2</sup> .	4. PLEASE REFER TO ARCHITECTS DRAWINGS FOR FINAL BUILDING LOCATION.
<ul> <li>26. BRICKS OR BLOCKS MUST NOT BE PLACED IN THE BEDDING MORTAR FOR SETTING THE PIPES TO LEVEL .</li> <li>27. ALL ROCKER PIPE LENGHTS TO BE MIN 600mm</li> </ul>	<ol> <li>THE LOCAL AUTHORITY AND SERVICE COMPANIES ARE TO BE NOTIFIED PRIOR TO COMMENCEMENT OF WORK ON SITE.</li> <li>ALL DRAINAGE COMPONENTS ARE TO COMPLY WITH CURRENT BRITISH STANDARDS AND BUILDING</li> </ol>
<ol> <li>27. ALL ROCKER FILE LENGITTS TO BE WIN OUTIGIN.</li> <li>28. PROVIDE ROCKER PIPES AT TRANSITION FROM CONCRETE SURROUND TO GRANULAR SURROUND.</li> </ol>	<ul><li>REGULATIONS REQUIREMENTS.</li><li>7. ALL WORKS TO BE IN ACCORDANCE WITH THE LOCAL</li><li>AUTHORITY'S ROADS FOR A DOPTION SPECIFIC ATION.</li></ul>
29. MAX DISTANCE FROM FACE OF CONCRETE SURROUND TO FIRST FLEXIBLE JOINT TO BE 150mm.	<ol> <li>ALL WORKS AND MATERIALS TO BE IN ACCORDANCE WITH THE SPECIFICATION FOR HIGHWAY WORKS (SHW SERIES 500).</li> </ol>
30. <u>MANHOLE COVERS AND FRAMES</u> MANHOLE COVERS AND FRAMES SHALL COMPLY WITH THE RELEVANT PROVISIONS OF THE BS EN 124,M BS 7903 AND HIGHWAYS AGENCY GUIDANCE DOCUMENT HA 104/09. THEY	9. DRAIN PIPE THROUGH WALLS OR BENEATH FOUNDATIONS (SPREAD ONLY) TO HAVE R.C BRIDGE LINTELS OVER AND PIPE SURROUNDED IN FLEXIBLE MATERIAL (50mm).
RELAY TO THE CUSHION INSERTS. MANHOLE COVER ON FOUL ONLY SEWERS SHALL BE OF LOW LEAKAGE TYPES IN ORDER TOP PREVENT EXCESSIVE	<ol> <li>FINAL LOCATIONS AND DETAILS OF SOIL VENT PIPES, STUB STACKS, RAINWATER DOWN PIPES, GULLIES ETC. TO BE CONFIRMED BY REFERENCE TO ARCHITECT' DRAWINGS.</li> <li>ALL THRESHOLD DRAIN DETAILS TO BE TO ARCHITECT'</li> </ol>
SURFACE WATER INGRESS AS A MINIMUM, CLASS D400 SHALL BE USED IN CARDIACEWAYS OF BOADS (INCLUDING REDESTRIAN	<ol> <li>ALL PIPES INTO CHAMBERS TO SOFFIT TO SOFFIT U.N.O.</li> </ol>
<ul> <li>31. CONSTRUCTION OF SEWER TO BE IN ACCORDANCE WITH WELSH MINISTERS STANDARDS AND SFA 7TH EDITION.</li> </ul>	13. AT ALL OUTFALL POINTS TO AN EXISTING NETWORK, THE POSITION AND INVERT LEVEL OF EXISTING DRAINS MUST BE CONFIRMED WELL IN ADVANCE OF THE PROGRAMMED DATE FOR INSTALLING ANY OF THE UPSTREAM DRAINAGE, OR
KEY	FOR ANY NECESSARY REVISIONS TO THE HYDRAULIC DESIGN.
DENOTES PROPOSED FOUL CHAMBER & PIPE RUN TO BE ADOPTED BY WELSH WATER. DENOTES PROPOSED FOUL CHAMBER & PIPE RUN	14. ALL GRAVITY UPVC PIPEWORK TO BE TO BS 4660 OR BS 5481 WHERE RELEVANT UNLESS NOTED OTHERWISE.
TO BE ADOPTED BY WELSH WATER.	15. ALL NON ADOPTABLE DOMESTIC FOUL AND SURFACE WATER PIPE RUNS SHALL CONSIST OF 100mm DIA. PIPES LAID AT NO FLATTER THAN 1/80 FALLS U.N.O. A SEWER OR LATERAL DRAIN WITH A NOMINAL INTERNAL DIAMETER OF
DENOTES PROPOSED SURFACE WATER CHAMBER     & PIPE RUN, TO BE ADOPTED BY THE SuDS     APPROVAL BODY (SAB)      RE     DENOTES PROPOSED RODDING EYE.	100mm, OR A LATERAL DRAIN SERVING TEN OR LESS PROPERTIES IS LAID TO A GRADIENT NOT FLATTER THAN 1:80, WHERE THERE IS AT LEAST ONE WC CONNECTED AND 1:40 IF THERE IS NO WC CONNECTED.
G1 DENOTES PROPOSED HIGHWAY GULLY AND Ø 150mm CONNECTION TO SURFACE WATER NETWORK TO BE ADIOPTED BY THE HIGHWAY AUTHORITY UNDER SECTION 38.OF THE HIGHWAYS ACT 1980.	16. ALL CONNECTIONS FROM HIGHWAY GULLIES TO BE 150mm DIA. LAID AT FALLS OF BETWEEN 1/20 AND 1/100 WITH TYPE S BED AND SURROUND TO ALL CONNECTIONS WITH MIN. 1.20m COVER, TYPE Z BED AND SURROUND TO ALL OTHER CONNECTIONS.
DENOTES PROPOSED SITE BOUNDARY. DENOTES PROPOSED POROUS PAVING TO BE LOACTED MIN 3m FROM ANY FOUL SEWER.	17. <u>THERMOPLASTIC PIPES &amp; FITTINGS:</u> THERMOPLASTIC PIPES, JOINTS & FITTINGS FOR GRAVITY SEWERS SHALL COMPLY WITH THE RELEVANT PROVISIONS OF BS EN 1401-1, BS EN 1852 & BS EN 12666-1.
WB DENOTES ABOVE GROUND WATER BUTT AT BASE OF RAIN WATER DOWN PIPE.	18. THERMOPLASTIC STRUCTURED WALL PIPE: THERMOPLASTIC STRUCTURED WALL SEWER PIPE SHALL COMPLY WITH THE RELEVANT PROVISIONS OF BS EN 13476-1
DC DENOTES DISH CHANNEL KERB TO CONVEY WATER FROM WATER BUTT INTO RAIN GARDEN. DENOTES PROPOSED RAIN GARDEN WITH PERFORATED PIPE TO INTERCEPT SURFACE	& WIS 4-35-01 AND BS EN 13476-2 OR BS EN 13476-3. PIPES SHALL BE BSI KITEMARKED OR HAVE EQUIVALENT THIRD PART CERTIFICATION. PIPES LESS THAN OR EQUAL TO 500mm IN DIAMETER SHALL HAVE NOMINAL SHORT-TERM RING STIFFNESS NOT LESS THAN 8KN/m <sup>2</sup> (SN8) OR BE SUBJECT TO A
WATER THROUGH INFILTRATION. DENOTES EXTENT OF 6M EASEMENT	Nom. SHORT TERM RING STIFFNESS OF 2KN/m <sup>2</sup> (SN2) IS ACCEPTABLE FOR PIPES GREATER THAN Ø500mm, SUBJECT TO SUPPORTING STRUCTURAL DESIGN LOAD CALCULATIONS
	BEING PROVIDED. TRANSPORTATION, HANDLING, STORAGE AND LAYING SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS.
	WHERE A FITTING IS INSTALLED ON A SEWER LENGTH, IT SHALL HAVE THE SAME INTERNAL BORE AS THE SEWER. Max. LENGTH OF PIPE FOR LAYING IS 3.0m OR Ø x 10, WHICHEVER IS THE GREATER, UNLESS WELDED JOINTS ARE
	<ul> <li>19. <u>CONNECTION TO THE PUBLIC SEWER</u></li> <li>A SECTION 106 APPLICATION TO CONNECT MUST BE MADE TO DCWW. THE DEVELOPER SHALL GIVE 21 DAYS NOTICE PRIOR TO CONNECTION, THE WORKS MAY ONLY BE UNDERTAKEN BY A DCWW HEALTH AND SAFETY APPROVED</li> </ul>
	<ul> <li>CONTRACTOR.</li> <li>21. <u>OPTIMUM TRENCH WIDTH</u> OPTIMUM TRENCH WIDTH = PIPE + 300mm. CONTRACTOR TO</li> </ul>
	<ul> <li>ENSURE TRENCH WALLS ARE SUITABLY PROPPED.</li> <li>22. BACKFILLING TO PIPE TRENCHES BENEATH ROADS, CAR</li> <li>BARKING AND STRUCTURES TO BE MOLT. TYPE 1 GRANULAR</li> </ul>
	MATERIAL UP TO FORMATION LEVEL FROM THE TOP OF THE SPECIFIED PIPE SURROUND (WELL COMPACTED IN 150mm LAYERS).
	S1         P01         21.06.22         FIRST ISSUE         I         I         I         I           SUITABILITY         REV         DATE         DESCRIPTION         Org.         Chk'd         App'd         Auth.
	PROJECT TITLE: LAND ADJ TO CROWN STREET, GWALCHMAI
	DRAWING TITLE: PROPOSED SECTION 104 DRAINAGE LAYOUT
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$
	ORIGINATOR:DATE:SCALE:ORIGINAL SIZE:B.Thorne21.06.20221:250A1
	CADARN
	CONSULTING ENGINEERS         Address:       CADARN Consulting Engineers Ltd, Suite B.
	Anglesey Business Centre, Bryn Cefni, Llangefni, <b>Tel:</b> 01407 730912 LL77 7XA <b>E-mail:</b> Admin@cadarnconsulting.co.uk



## **APPENDIX H Proposed Impermeable Area Plan**



			FFFFCTIVE	NOTES
	TOTAL AREA	COEFFICIENT	AREA	1. DO NOT SCALE FROM THIS DRAWING.
	1,646.241 m <sup>2</sup>	1.00	1,646.241 m <sup>2</sup>	2 ALL LEVELS IN METRES UNLESS NOTED OTHER WISE
	864.832 m <sup>2</sup>	1.00	864.832 m <sup>2</sup>	ON DRAWING.
	1,401.139 m <sup>2</sup> 1,189.051 m <sup>2</sup>	1.00	1,401.139 m <sup>2</sup> 1.189.051 m <sup>2</sup>	3. ALL DIMENSIONS AND LEVELS TO BE CHECKED ON
	2615.401 m <sup>2</sup>	0.35	915.390 m <sup>2</sup>	SITE PRIOR TO UNDERTAKING ANY WORKS, ORDERING MATERIALS OR FABRICATING ANY
	429.651 m <sup>2</sup>	1.00	429.651 m <sup>2</sup>	COMPONENTS.
	8,206.316 m <sup>2</sup>		6506.304 m <sup>2</sup>	4. THIS DRAWING IS TO BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT ENGINEER'S AND
				ARCHITECT'S DRAWINGS AND RELEVANT
			-	SI ECIFICATION CLAUSES.
			_	KEY
				DENOTES AREA OF PROPOSED GRASS.
				DENOTES AREA OF PROPOSED HOUSES.
				DENOTES AREA OF PROPSOED PARKING.
				DENOTES AREA OF PROPOSED CONCRETE PATHS.
				DENOTES AREA OF PROPOSED ROAD.
			$\times$	DENOTES AREA OF PROPOSED SWALE.
				DENOTES SITE BOUNDARY
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				S1         FUI         20.06.22         FIRST ISSUE           SUITABILITY         REV         DATE         DESCRIPTION         Org.         Chk'd         App'd         Auth.
				DRAWING STATUS: PROJECT TITLE:
				LAND ADJ TO CROWN STREET,
				GWALCHMAI
				DRAWING TITLE:
				PROPOSED AREAS
				DRAWING No:
				PROJECT ORIGINATOR VOL. LOC. TYPE ROLE
				U9422 CCE V1 XX 40:40:01 C
				JU.JU     UUU4     JI     FU1       CLASSIFICATION     No.     SUITABILITY     REVISION
				ORIGINATOR:     DATE:     SCALE:     ORIGINAL SIZE:       B Thorma     20.06.2022     1.250     4.1
				D. THOME 20.00.2022 1:230 AI
				CONSULTING ENGINEERS
				Address: CADARN Consulting Engineers Ltd,
				Suite B, Anglesey Business Centre,
				Bryn Cefni, Llangefni, Lla7 77 1 Tel: 01407 730912
				LL///XA E-mail: Admin@cadarnconsulting.co.uk